## G-III6

## A Simple Mathematical Model for Sediment Transport Franz EBERSOLDT and Michael EHMKE

Fachbereich Mathematik (FB 11), Universität Duisburg, Lotharstr. 61, 4100 Duisburg (F.R.G.)

Sediment transport in water is mostly studied by using purely empirical methods. But in order to predict sedimentary deposit or erosional effects, one would like to have some theoretical tools for the description of sediment motion.

We consider a two-dimensional (x,z) stationary model, where the main sediment flow is horizontally (x-axis) and where the ground has a distance z = h(x) from the surface.

z = h(x) from the surface. Using diffusion theory one derives - with some physically justified simplifi-cations - from the continuity equation (sedimentary mass conservation) the following parabolic partial differential equation for the mean sediment con-centration in suspension c(x, z)

$$u\frac{\partial c}{\partial x} + (w - w_s)\frac{\partial c}{\partial z} - \frac{\partial}{\partial z}(\epsilon_s\frac{\partial c}{\partial z}) = 0$$
(1)

with some appropriate initial and boundary conditions. u and w are the x resp. z-components of the velocity field of the current. The coefficient  $\epsilon$ , takes into account the sedimentary exchange due to tur-bulent flux. Using a lot of experimental data KERSSENS [2] gave a suitable

formula for  $\epsilon_{\ell}(z)$ . The constant  $w_{\epsilon}$  characterizes the mean sedimentation speed, which is a function of the diameter, form and mass of the sediment particles and of the

The mean velocity field (u,w) could of course be determined by solution of the associated time-independent Navier-Stokes equations for a viscous incompressible fluid. Since they are valid only for a laminary current, one could superpose a turbulent motion in adding a term taking into consideration the

superpose a turbulent motion in adding a centricating into consideration the Reynolds tensions and the laminary layer phenomena. Instead of choosing this complicated approach necessitating the resolution of a nonlinear system of partial differential equations, we follow an idea of PRANDTL [3] for the description of the shear-stresses. Introducing a rough-ness function for the description of the ground structure, as it has been done by J. NIKURADSE (see [4]), one can find by elementary integrations that

$$u = \frac{K \ln(z/z_0)}{\ln(h/z_0) - 1}$$
(2)

and

$$w = \frac{K \ln(h/z_0) \frac{dh}{dx}}{h \ln(h/z_0) - 1)} (z \ln(z/z_0) - z + z_0), \tag{3}$$

where the constants K and  $z_0$  depend on h(x) and the roughness of the ground.

If one uses (2) and (3), the diffusion equation (1) can be solved numeri-cally by standard discretisation techniques.

Knowing the local sedimentary density c(x, z) and the current field (u, w), one easily calculates the total transport of the suspended particles.

Beyond that the motion of drift materials can be treated by the Engelund-Hansen method [1], which is also applied for finding the necessary boundary conditions at the ground level.

## REFERENCES.

ENGELUND, F.; HANSEN, E., "A Monograph on Sediment Transport in Alluvial Streams", Kopenhagen 1967

[2] KERSSENS, P.J.M., "New Developments in Suspended Sedi-ment Research", Delft Hydraulic Laboratory 237/1980

[3] PRANDTL, L., "Über die ausgebildete Turbulenz", Z. Ang. Math. Mech. 5/1925, 136-139

[4] SCHLICHTING, H., "Grenzschicht-Theorie", Karlsruhe 1965